TECHNICAL MEMORANDUM

Client:	City of Sequim
Project:	General Sewer Plan Update
Project File:	SEQM 0240176.00.0016
Project Manager:	Zach Schrempp, PE, and Eric Smith, PE
Composed by:	Edgar Daniel and Zach Brodkey
Reviewed by:	Dan Mahlum, PE
Subject:	Capacity Analysis for Capital Facilities Plan
Date:	May 15, 2026



Signed: 05/15/2026



Signed: 05/15/2026

Introduction

The City of Sequim's (City) sanitary sewer utility provides sanitary sewer service within most of the City limits and the surrounding Urban Growth Area (UGA). Sanitary sewer service is also provided to an unincorporated area of Bell Hill to the south of the City and several customers to the southeast of the City (Sequim Bay Lodge, Sequim Bay State Park, and the Jamestown S'Klallam Tribe). While sanitary sewer services to Carlsborg are provided by Clallam County's Utilities Division, the City accepts and treats wastewater from Carlsborg. RH2 Engineering, Inc., (RH2) is currently in the process of updating the City's *General Sewer Plan* (GSP), which contains (among other elements) a detailed sewer system description, flow and loading projections, collection and treatment system analysis, a Capital Improvement Plan (CIP), and a financial analysis.

The existing sewer collection system contains approximately 71.8 miles of gravity mains, 4 lift stations, and 6.5 miles of force mains that convey wastewater to the City's Water Reclamation Facility (WRF). Additional collection system components tributary to the City system are owned by others. The WRF treats wastewater to produce Class A reclaimed water that is either used by the City for approved uses or discharged via an outfall in the Strait of Juan de Fuca.

The City asked RH2 to prepare this technical memorandum summarizing the existing and projected capacity of the sanitary sewer system in support of the Capital Facilities Plan section of the City's 2025 *Comprehensive Plan* update.

Existing System Flow and Loading

Existing (2025) flow and loading for the collection system were developed by analyzing historical influent flow and load data found in the WRF's Discharge Monitoring Reports (DMR). An average of the data from the past 4 years (2022 to 2025) was used to represent existing conditions, as there has not been a distinct increase in flow or loading over this period, and the 2025 flow and loading were slightly lower than those for 2022 and 2024. The existing flow parameters determined include average annual, maximum monthly, and peak hour flows. The historical average annual and maximum monthly flow and loading are based on publicly available DMR data. The potential WRF peak hour flow is estimated based on the typical peaking factor of 4 relative to the average annual flow to conservatively estimate peak hydraulic conditions in the collection system. The existing flows were apportioned to individual areas of the collection system based on the distribution of residential customers among the areas. The existing loading parameters determined include average annual and maximum monthly organic loading and solids loading, measured by 5-day biochemical oxygen demand (BOD_5) and total suspended solids (TSS) in the influent flow.

The City is currently initiating an infiltration and inflow (I/I) study to characterize the I/I component of existing flows.

Projected System Flow and Loading (2045)

Projected wastewater flow and loading rates have been developed for the 20-year planning period used in the GSP. The sewer service area is expected to experience population growth during this period, which will lead to increased wastewater flow and loading in the collection and treatment system.

Projected 2045 flow and loading were developed for this technical memorandum by first estimating a baseline of dry-weather residential flows to the WRF and dry-weather residential per capita flow. Dry-weather flows are used as the basis for projections to exclude the influence of I/I. The estimated dry-weather per capita flow was used for all existing residents served, and additional residents to be served were assigned a greater per capita flow value for conservatism. For each year, the per capita flow value was multiplied by the population projection to determine the projected baseline residential flow. The population projection includes City limits and UGA, Bell Hill, and Carlsborg. Details on the population projection methodology, including the assumptions used and the guidance for calculating these projections, are included in **Attachment 1**.

To estimate baseline flow of customers along Highway 101, specifically those in the Blyn limited area of more intensive rural development, which the City serves through a wholesale sewer agreement with the Jamestown S'Klallam Tribe, historical influent data from the force main serving these customers were analyzed to determine their annual average and maximum

monthly flows from 2022 to 2025. These values were projected forward at the same growth rate as the population within the City. The sum of baseline dry-weather residential flow and baseline Highway 101 customer flow represents the baseline total dry-weather flow used for projecting other flow conditions. The baseline total dry-weather flow was projected forward using the population projections for the residential component and the City growth rate for the Highway 101 customer component.

For average annual and maximum monthly flow conditions, a peaking factor was produced by dividing the existing value by the existing total dry-weather value. Then, for each year in the projections, the respective peaking factors were used to determine projected average annual and maximum monthly flow values from the baseline total dry-weather flow projection. Peak hour flows were then estimated for each year using a standard peaking factor of 4 relative to the average annual projections.

Loading projections did not consider dry-weather conditions since organic and solids loading should not be affected by I/I, and did not separate residential from non-residential sources due to the relative insignificance of flow from these sources and the lack of loading data specific to these sources. Per capita BOD_5 and TSS loading values were determined based on the existing average annual loading and residential population served. Average annual loading projections were estimated by multiplying the projected service population by the existing per capita loading. For maximum monthly loading conditions, peaking factors were produced by dividing the existing maximum monthly values by the existing average annual values. Then, for each year in the projections, the respective peaking factors were used to determine projected maximum monthly loading from the average annual loading projections. The flow and loading projections produced using this methodology are shown for each area in **Table 1**.

Table 1
Sewer System Flow and Loading

Year	AAF (MGD)	MMF (MGD)	PHF (MGD)	BOD_5 AA (ppd)	BOD_5 MM (ppd)	TSS AA (ppd)	TSS MM (ppd)
City Limits							
2025	0.51	0.57	2.03	1,441	1,619	1,413	1,645
2045	0.84	0.95	3.36	2,280	2,561	2,236	2,602
Outside City Limits within UGA							
2025	0.03	0.04	0.13	89	100	88	102
2045	0.17	0.19	0.66	426	479	418	487
Bell Hill							
2025	0.05	0.05	0.19	136	153	133	155
2045	0.08	0.09	0.31	219	246	215	250
Carlsborg							
2025	0.04	0.05	0.16	117	131	115	134
2045	0.09	0.10	0.38	250	281	245	285
Flow and Loading East of Sequim							
2025	0.02	0.03	0.09	< 100	< 100	< 100	< 100
2045	0.03	0.03	0.12	< 100	< 100	< 100	< 100
Totalized Values to WRF							
2025	0.65	0.73	2.59	1,783	2,003	1,749	2,036
2045	1.20	1.36	4.82	3,175	3,567	3,113	3,624
WRF Design Criteria							
		1.67 MGD			4,036 ppd		3,855 ppd

AAF = average annual flow
MGD = million gallons per day
MMF = maximum monthly flow
PHF = peak hour flow
AA = average annual
ppd = pounds per day
MM = maximum monthly

Comparison of Existing and Projected Flow and Loading to WRF Capacity

The existing flow and loading to the WRF is approximately 50 percent of the permitted capacity. The 20-year projections for 2045 predict that flow and loading to the WRF will near but not exceed the permitted capacity. However, the flow, organic loading, and solids loading are projected to surpass 85 percent of their permitted capacities in 2046, 2044, and 2042, respectively. The WRF's National Pollutant Discharge Elimination System Permit stipulates that if either flow or load reaches 85 percent of their permitted capacity for 3 consecutive months, then the City must begin planning for expansion of WRF capacity. Therefore, the City should

expect to begin planning for capacity expansion near the end of the planning period by completing an Engineering Report in accordance with Washington Administrative Code (WAC) 173-240-060. This may include major expansion and/or modification to the biological treatment system, such as construction of an additional aeration basin and a fourth secondary clarifier.

The WRF solids handling system will undergo significant upgrades during the planning period, primarily related to increasing the reliability of the system and replacing the Class A biosolids treatment system. An Engineering Report is currently being prepared for the City that details the recommended strategy to expand the aerobic digestion system and replace the aging Class A biosolids treatment system in a two-phase project.

Various systems and components throughout the WRF will require replacement during the planning period due to their age and condition. These improvements are generally expected to include the following:

- Refurbishment of major equipment items:
    - Repair and recoating existing clarifier mechanisms and replace drive units.
- Replacement of equipment and components at the end of their useful life may include, but not be limited to, the following:
    - Replace one headworks screen.
    - Replace grit system components.
    - Replace equalizer system mixers and pumps.
    - Replace aeration basin diffusers and other components.
    - Replace return activated sludge, waste activated sludge, and scum pumps.
    - Replace effluent lift station pumps.
    - Replace flocculation tank mixers.
    - Replace reclaimed water anthracite filter internals, control panel, and supporting equipment.
    - Replace backwash supply and waste pumps,
    - Replace the ultraviolet system.
    - Replace reclaimed water lift station pumps.
    - Replace non-potable water pumps.
    - Replace generator, motor control, control panel, and other electrical and control equipment.

Existing Lift Station Capacity

The existing capacity of the City's lift stations is shown in gallons per minute (gpm) in **Table 2**. Within the City limits, existing peak hour flows were calculated on a Transportation Analysis Zone (TAZ) basis and then spatially allocated to maintenance holes within their respective TAZ boundaries. The peak hour flows within the Bell Hill area were identified by address and allocated to the nearest maintenance hole. The summation of flow tributary to each lift station was utilized to estimate the remaining lift station capacity.

Table 2
Existing (2025) Lift Station Capacity

Lift Station Name	Location	No. of Pumps	Type	Manufacturer	Horsepower (each)	TDH (feet)	Total Capacity (gpm)	Firm Capacity (gpm)	PHF Tributary to LS (gpm)	Remaining Capacity (gpm)
Doe Run	Doe Run Road & Fox Hollow Road	2	Dry Pit Non-clog centrifugal	Smith and Loveless	25.0	162	200	100	34	66
Port Williams	N. Brown Road & Harriette Lane	2	Non-Clog Centrifrugal	Cornell	20.0	108	700	350	227	123
Cedar Ridge	Lofgrin Road & Ida Court	2	Wet-pit, non-clog, Submersible	Wilo-EMU USA	10.0	82	432	216	2	214
Jeannie's Meadow	East Lobelia	2	Wet-pit, non-clog, Submersible	Wil-EMU USA	23.5	135	330	165	17	148

TDH = total dynamic head
LS = lift station

Projected Lift Station Capacity (2045)

The projected remaining capacity of the City's lift stations in 2045 is shown in **Table 3**. Flow tributary to each lift station were estimated utilizing the same methodology described in the **Existing Lift Station Capacity** section. The projected lift station capacity analysis identifies that the Port Williams Lift Station will be out of capacity by 2045. To accommodate anticipated growth, the City plans to construct a new lift station east of town near Forrest Road. The firm capacity of all other lift stations can support future growth. Upgrades for the Port Williams Lift Station and construction of the Forrest Road Lift Station are included in the **Capital Improvement Projects** section in this technical memorandum.

Table 3
Projected (2045) Lift Station Capacity

Lift Station Name	Location	No. of Pumps	Type	Manufacturer	Horsepower (each)	TDH (feet)	Total Capacity (gpm)	Firm Capacity (gpm)	PHF Tributary to LS (gpm)	Remaining Capacity (gpm)
Doe Run	Doe Run Road & Fox Hollow Road	2	Dry Pit Non-clog centrifugal	Smith and Loveless	25.0	162	200	100	65	35
Port Williams	N. Brown Road & Harriette Lane	2	Non-Clog Centrifrugal	Cornell	20.0	108	700	350	527	-177
Cedar Ridge	Lofgrin Road & Ida Court	2	Wet-pit, non-clog, Submersible	Wilo-EMU USA	10.0	82	432	216	4	212
Jeannie's Meadow	East Lobelia	2	Wet-pit, non-clog, Submersible	Wil-EMU USA	23.5	135	330	165	25	140

Existing Collection System Flows

A collection system model is currently being developed to characterize the capacity of the sewer collection system (gravity collection system, lift stations, and force mains). As directed by

the City, the collection system model contains only the most important trunklines within the collection system.

Figure 1 shows the existing collection system flows. Existing peak hour flows were incorporated into the hydraulic sewer model using methods tailored to the five areas of interest identified in **Table 1**. Within the City limits and the UGA, flows were calculated on a TAZ basis and spatially allocated equally to maintenance holes located within or nearest to each TAZ. Peak hour flows for the Bell Hill area were calculated on a per address basis and assigned to the maintenance hole(s) closet to each address. The remaining two areas of interest, Carlsborg and the area east of Highway 101, discharge directly into the City's sewer infrastructure. The calculated flows for these areas were assigned to the maintenance holes that receive their respective discharges.

The total flow shown at the WRF in **Figure 1** reflects modeled flows rather than the peak hour flow totals listed in **Table 1**. In the model, lift station pumps generate the hydraulic grade needed to move wastewater; the corresponding pump flows are larger than what is assigned to the tributary maintenance holes. Although the modeled WRF flow is higher than the calculated value, it provides a more conservative representation of the flows in the collection system.

Under existing system conditions, maintenance hole 20-1-19, highlighted in **Figure 1**, was identified as a capacity concern. The hydraulic model indicates that this maintenance hole, located near North Blake Avenue and East Willow Street, is surcharging. While the City has not observed any physical surcharging within this area, this finding is consistent with the City's 2013 GSP, which also identified surcharging at this location under the 2032 analysis. Improvements for this area are identified as CIP CS-13 within the 2013 GSP. Because no physical surcharging has been documented to date, the City has not prioritized CIP CS-13 for capital improvement.

Projected Collection System Flows (2045)

The projected system flows for 2045 are shown in **Figure 2**. The projected peak hour flows from **Table 1** were incorporated into the hydraulic model using following methodology: within the City limits, projected flows were calculated on a TAZ basis and allocated to the nearest maintenance holes; for the remaining areas of interest, the 2025 peak hour flows were scaled to match the projected 2045 totals for each area. The total flow shown at the WRF in **Figure 2** reflects modeled flows and an increase in the Port Williams Lift Station pumping capacity.

Areas of concern for capacity, including maintenance holes, are highlighted in **Figure 2**. The model identified several maintenance holes along North Blake Avenue, East Willow Street, West Washington Street, near West Salal Place, and near West Hammond Street as surcharging. These results are consistent with the 2013 GSP build-out analysis.

Capital Improvement Projects

The City is expecting population growth in the sewer service area by 2045. The projects shown in **Figure 3** are recommended to meet the projected growth of the system. New gravity mains, lift stations, and force mains are expected to be constructed as the collection system is extended to currently unserved parts of the service area. Collection system capacity

improvements (upsizing) will be required in several areas to accommodate the projected flow rates.

The results of the I/I study will also highlight areas where collection system rehabilitation projects are needed to reduce the I/I component of flow in the collection system. More detail on the effects of I/I within the sewer system will be included as part of the GSP.

Project List

A project list detailing pipe upsizing, capacity upgrades (WRF and lift stations), and projects to support system needs to accommodate projected growth within the City limits and UGA is shown in **Table 4**.

Table 4

Project List

Project No.	Project Description	Cost Estimate
	2026 to 2032 (+ 6-year)	
	WRF Improvements (Non-Growth Driven)	
WRF-1	Phase 1 Solids Handling Improvements Design (Non-growth driven).	$1.0M
	2033 to 2045 (7- to 20-year)	
	WRF Improvements	
WRF-2	Complete Engineering Report of future activated sludge system improvements as facility loading nears 85% of design capacity.	-
WRF-3	Construct Phase 1 Solids Handling Improvements to expand aerobic digestion.	-
WRF-4	Design and construct Phase 2 Solids Handling Improvements to replace the Class A solids treatment system.	-
WRF-5	Existing Equipment Replacement or Refurbishment (multiple projects).	-
	Collection System Improvements	
CS-1	Upsize Grant Road to West Washington Street Gravity Main	-
CS-2	Upsize West Hammonds Gravity Main	-
CS-3	Upsize Sewer Gravity Main near West Salal Place	-
CS-4	Upsize Sewer Gravity Main Along North Blake Avenue	-
	Lift Station Improvements	
LS-1	Forrest Road Lift Station and Force Main	-
LS-2	Port Williams Lift Station Improvements	-

Conclusion

Based on the analyses being performed as part of the City's GSP update, sewer system improvements, including gravity and force mains, lift stations, and WRF systems, will be necessary to meet anticipated growth and projected flow and loads. As shown in **Table 1**, peak hour flows are estimated to increase by approximately 66 percent by 2045. Over that same period, BOD_5 and TSS flows and loading are estimated to increase by approximately 58 percent each. The WRF is also expected to reach 85 percent of its permitted capacity by 2042. The City is currently planning for a large, two-phased project to increase the reliability of the solids handling system and ultimately replace the aging Class A biosolids treatment system. Additional improvements will be necessary near the end of the planning period to expand WRF capacity to meet the projected flow and loads.

While the City's lift stations currently have surplus capacity to handle peak flows, the Port Williams Lift Station will need upgrades before the end of the 20-year planning period to have enough capacity to meet projected peak flows. New gravity mains, lift stations, and force mains are expected to be constructed as the collection system is extended to currently unserved parts of the service area. These projects are summarized in **Table 4** and will be included in the City's GSP update with greater detail in scope and cost.

Attachments

Figure 1 – 2025 System Peak Hour Flows and Areas of Concern
Figure 2 – Future 2045 System Peak Hour Flows and Areas of Concern
Figure 3 – Growth-Related Capital Improvement Projects
Attachment 1 – Population Projection Methodology

Figures

Port Williams
Lift Station
No. of Pumps: 2
Capacity: 350 (gpm/pump)

Jeannie's Meadow
Lift Station
No. of Pumps: 2
Capacity: 165 gpm/pump

City of Sequim WRF
Total Modeled
Total Flow Received:
3.15 MGD

Area of Concern

Cedar Ridge
Lift Station
No. of Pumps: 2
Capacity:
216 gpm/pump

Force Main from
Carlsborg
Collection System
Total Flow Received:
0.16 MGD

City Limits
Total Flow Received:
2.03 MGD
UGA
Total Flow Received: 0.13

Bell Hill Area
Total Flow Received:
0.19 MGD

Doe Run
Lift Station
No, of Pumps: 2
Capacity: 100 gpm/pump

Force Main from
Jamestown, West Sequim Bay Lodge,
Seven Cedars Casino & S'Klallam Tribe
Collection System
Total Flow Received: 0.09 MGD

Dungeness River

Sequim Bay

Legend

- City Limits
- City of Sequim UGA

Sewer System Infrastructure

- WRF
- Lift Station
- City Sewer Maintenance Hole
- City Sewer Main

Modeled Infrastructure

- Modeled Maintenance Hole
- Model Sewer Main
- Force Main

Areas of Concern

- Maintenance Hole Surcharging

Sewer Drainage Basins

- 1
- 1a
- 1b
- 2
- 3
- 4
- 5
- 6
- 6a
- 7
- 8
- 9

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Vicinity Map

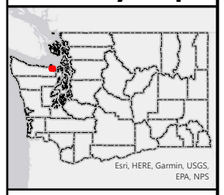


Figure 1
2025 System Peak Hour Flows and Areas of Concern
City of Sequim Comprehensive Plan

CITY OF SEQUIM

1 inch : 1,000 Feet

0 500 1,000 2,000 Feet

DRAWING IS FULL SCALE
WHEN BAR MEASURES 2"

RH2

NORTH

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Port Williams
Lift Station
No. of Pumps: 2
Capacity: 600 (gpm/pump)

Jeannie's Meadow
Lift Station
No. of Pumps: 2
Capacity: 165 gpm/pump

City of Sequim WRF
Total Modeled
Total Flow Received:
5.20 MGD

Cedar Ridge
Lift Station
No, of Pumps: 2
Capacity:
216 gpm/pump

Force Main from
Carlsborg
Collection System
Total Flow Received:
0.38 MGD

City Limits
Total Flow Received:
3.36 MGD
UGA Total Flow Received:
0.66 MGD

Bell Hill Area
Total Flow Received:
0.31 MGD

Doe Run
Lift Station
No, of Pumps: 2
Capacity: 100 gpm/pump

Force Main from
Jamestown, West Sequim Bay Lodge,
Seven Cedars Casino & S'Klallam Tribe
Collection System
Total Flow Received: 0.12 MGD

Dungeness River

Sequim Bay

Legend

- City Limits
- City of Sequim UGA

Sewer System Infrastructure

- WRF
- Lift Station
- City Sewer Maintenance Hole
- City Sewer Main

Modeled Infrastructure

- Modeled Maintenance Hole
- Model Sewer Main
- Force Main

Areas of Concern

- Maintenance Hole Surcharging
- Sewer Main of Concern

Sewer Drainage Basins

1, 1a, 1b, 2, 3, 4, 5, 6, 6a, 7, 8, 9

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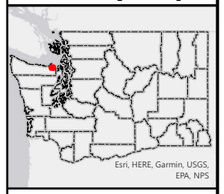


Figure 2
Future 2045 System Peak Hour Flows and Areas of Concern
City of Sequim Comprehensive Plan

CITY OF SEQUIM

1 inch : 1,000 Feet

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DRAWING IS FULL SCALE
WHEN BAR MEASURES 2"

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NORTH

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Figure 3
Growth Related Capital Improvement Projects
City of Sequim Comprehensive Plan

Port Williams Lift Station
No. of Pumps: 2

Jeannie's Meadow Lift Station
No. of Pumps: 2
Capacity: 165 gpm/pump

City of Sequim WRF

Forest Road Lift Station

Cedar Ridge Lift Station
No, of Pumps: 2
Capacity: 216 gpm/pump

Force Main from Carlsborg Collection System

Doe Run Lift Station
No, of Pumps: 2
Capacity: 100 gpm/pump

Force Main from Jamestown,West Sequim Bay Lodge, Seven Cedars Casino & S'Klallam Tribe Collection System

Dungeness River

Sequim Bay

Legend

- City Limits
- City of Sequim UGA

Sewer System Infrastructure

- WRF
- Lift Station
- City Sewer Maintenance Hole
- City Sewer Main

Modeled Infrastructure

- Modeled Maintenance Hole
- Model Sewer Main
- Force Main

Growth Related Projects

- Lift Station Improvements
- Maintenance Hole Improvements
- Force Main Projects

Sewer Drainage Basins

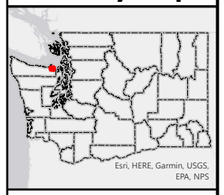
1, 1a, 1b, 2, 3, 4, 5, 6, 6a, 7, 8, 9

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DRAWING IS FULL SCALE WHEN BAR MEASURES 2"

RH2

NORTH

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Attachment 1

Population Projection Methodology

City of Sequim
Capacity Analysis for Capital Facilities Plan Technical Memorandum
Population and Employment Methodology

Area	Data (Sources)	Methodology
City of Sequim (inside the City limits)	- **2023 TAZ dataset** . (City of Sequim 10/16/2025) - **2045 City of Sequim growth allocations** . 2045 TAZ dataset (City of Sequim 11/21/2025) . Clarifications (City of Sequim 3/25/2026) - **Clallam County septic system status GIS shapefile** . (Clallam County GIS Portal 10/24/2025) - **Persons per household ratio** . (Leland Consulting 10/7/2025)	-Utilize 2045 GMA growth projections as allocated by Clallam County and as provided by the City - Use GIS to spatially proportion 2023 and 2045 TAZ data to the area within the City limits - Utilize GIS and the County's septic system status to determine what percentage of the City is sewered - A constant growth rate percentage will be calculated between the existing 2023 and future 2045 population - Estimate interim years assuming a constant growth rate percentage per TAZ - Convert households to population using person per household ratio
City of Sequim UGA (outside of the City limits)	- **2023 TAZ dataset** . (City of Sequim 10/16/2025) - **2045 City of Sequim Urban Growth Areas growth allocations** . (City of Sequim 3/25/2026) - **Clallam County septic system status GIS shapefile** . (Clallam County GIS Portal 10/24/2025) - **Persons per household ratio** . (Leland Consulting 10/7/2025)	- Use GIS to spatially proportion 2023 TAZ data to the area within the UGA but outside the City limits - Utilize GIS and the County's septic system status to determine what percentage of the City is sewered - A constant growth rate percentage will be calculated between the existing 2023 and future 2045 population - Estimate interim years assuming a constant growth rate percentage - Convert number of households to population using person per household ratio
Bell Hill	- **2023 Number of Connections** . (City of Sequim 12/11/2025) - **2045 Build out connections** . (City of Sequim 12/11/2025) - **County person per household ratio** . (2024 Clallam County Demographic Profile 10/20/2025)	- Use GIS to spatially allocate data to the Bell Hill area - Convert households to population using person per household ratio - A constant growth rate percentage will be calculated between the existing 2023 and future 2045 population
Carlsborg	- **2020 and 2045 population estimates** . (2025 Clallam County Land Capacity Analysis 10/16/2025) - **Clallam County septic system status GIS shapefile** . (Clallam County GIS Portal 10/24/2025)	- Utilize GIS and the County's septic system status to determine what percentage of Carlsborg is sewered - A constant growth rate percentage will be calculated between the historical 2020 and future 2045 population - Estimate interim years assuming a constant growth rate percentage - Assume the entire Carlsborg UGA will be sewered by the 20-year projection period
Highway 101 Customers	- **Historical force main flows** . (City of Sequim 12/08/2025)	- Population and employment estimates will not be made for this area. Historical flow data will be used to estimate existing flows, and future flows will be estimated per below. - Per directions from the City, apply the City's growth rate percentage to estimate future flows from Jamestown S'Klallam Tribe campus located at the Blyn Limited Area of More Intensive Rural Development (LAMIRD)

UGA = Urban Growth Area